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Section:	109.07.02 Fuel
Revision:	Change the Fuel/Work ratio for the items listed below:
	<u>Item Threshold Quantity Fuel/Work</u>
	Drainage Blanket, Asphalt Treated 5,000 tons 0.75
	Asphalt Mixtures for
	Pavements or Shoulders 3,000 tons 0.75
Section:	214.03 CONSTRUCTION
Revision:	Add the following as the final paragraph in the section:
	Demonstrate to the Engineer that the placement technique prevents damage to the fabric.
Section:	214.03 CONSTRUCTION
Subsection:	214.03.03 Slope Protection and Channel Lining
Revision:	Replace the first paragraph with the following:
	Place geotextile fabric for slope protection / geotextile fabric for channel lining with the long
	dimension parallel to the channel or toe of slope.
Section:	214.03 CONSTRUCTION
Subsection:	214.03.04 Underdrains
Revision:	Replace the first sentence in the subsection with the following:
	Place and shape geotextile fabric for subsurface drainage to the sides and bottom of the trench
	without stretching the fabric.
Section:	214.03 CONSTRUCTION
Subsection:	214.03.05 Subgrade or Embankment Foundation Stabilization
Revision:	Rename the subsection as follows: Subgrade Stabilization / Rock Roadbed
Section:	214.03 CONSTRUCTION
Subsection:	214.03.05 Subgrade Stabilization / Rock Roadbed
Revision:	Replace the first (1st) paragraph of the subsection with the following:
	Place geotextile fabric for stabilization, unless otherwise noted. Install with the long dimension
	parallel to the long dimension of the area to be covered.
Section:	214.03 CONSTRUCTION
Subsection:	214.03.05 Subgrade Stabilization / Rock Roadbed
Revision:	Add the following as the final paragraph in the section:
	Place, spread, and compact rock or backfill in such a manner that minimizes the development of wrinkles and movement in the fabric. In curves and intersections, cut the fabric and overlay appropriately. Keep the turning of tracked vehicles to a minimum to prevent displacement of the fill and damage to the fabric. Repair any damage caused during placement or by vehicles.

G	214 02 CONCEDITORIONI
Section:	214.03 CONSTRUCTION
Subsection:	214.03.06 Drainage Blanket
Revision:	Replace the first sentence in the subsection with the following:
	Place geotextile fabric for subsurface drainage with the long dimension parallel to the long
_	dimension of the area to be covered.
Section:	214.03 CONSTRUCTION
Subsection:	214.03.07 Embankment Foudation Working Platform
Revision:	Add the following as new subsection 214.03.07:
	214.03.07 Embankment Foundation Working Platform. To facilitate embankment
	construction over soft ground, place geotextile fabric for separation unless otherwise specified.
	Place as directed in the plans or by the Engineer. Install with the long dimension parallel to the
	long dimension of the area to be covered. Leave surface vegetation in place.
	During back dumping and spreading, do not allow the wheels of trucks, dozer blades, and other equipment to come into direct contact with the fabric. Spread the material in the direction of the
	fabric overlap. To avoid damage to the geotextile fabric, dump rock fill behind the leading edge
	of the rock layer, then blade into place. Repair any damage caused during placement or by
	vehicles. If large fabric wrinkles develop during spreading operations, fold and flatten the
	wrinkles in the direction of spreading. Avoid large folds which reduce the fabric overlap width.
Section:	214.05 PAYMENT
Revision:	Remove the following from list of pay items:
	02596-02599 Fabric-Geotextile, Type Square Yard
Section:	214.05 PAYMENT
Revision:	Add the following to the list of pay items:
	02602 Fabric-Geotextile Class 1 Square Yard
	02603 Fabric-Geotextile Class 2 Square Yard
Section:	215.02 MATERIALS
Subsection:	215.02.01 Geotextile Fabric
Revision:	Replace the text in this subsection with the following:
	Conform to Section 843.
Section:	215.03 CONSTRUCTION
Revision:	Replace the second sentence in the final paragraph with the following:
	Place a protective ring using geotextile fabric for subsurface drainage and separation; clean No.
	2 aggregate or shot rock of similar size, quality, and gradation approved by the Engineer; and
	crushed aggregate.
Section:	215.05 PAYMENT
Revision:	Remove the following from list of pay items:
	02596-02599 Fabric-Geotextile, Type Square Yard

Section:	215.05 PAYMENT						
Revision:	Add the following to the list of pay items:						
	02602 Fabric-Geotextile Class 1 Square Yard						
	02603 Fabric-Geotextile Class 2 Square Yard						
Section:	402.05.02 Asphalt Mixtures, HMA and WMA, Including Mixtures with Reclaimed Material						
Revision:	Replace the last sentence	sentence in this section with the following:					
	Each lot pay value will be	be averaged to dete	rmine the	final ove	rall bid item pay.		
Section:							
	402.05.02 Asphalt Mixto	ures, HMA and WI	MA, Inclu	ıding Mix	tures with Reclaimed Material		
Part:	LOT PAY ADJUSTME	NT SCHEDULE C	COMPAC	TION OP	TION A BASE AND BINDER		
	MIXES						
Table:	AV						
Revision:	Replace the table with the	ne following:					
	Г	A	V				
	l 1	Pay Value	Test	Result			
		Tay value	1115	%)			
			AADTT	AADTT			
			Class 2	Class 3			
		1.05	2220	or 4			
	1	1.05	3.2-3.8	3.2-3.8			
		1.00 + 0.1 (AV-3.0)	1.5-3.1	2.0-3.1			
		1.00 + 0.1 (4.5-AV)	3.9-6.0	3.9-6.0			
	1	0.75	6.1-6.5				
		0.75	0.1-0.5				
	1	(1)	< 1.5 or	< 2.0 or			
		1-7	< 1.5 OI	2.0 01	1		

Section:					
	402.05.02 Asphalt Mixtures, HMA and V	VMA, Inc	luding Mi	xtures with Reclaime	d Mater
Part:	LOT PAY ADJUSTMENT SCHEDULE	COMPA	CTION O	PTION A SURFACE	MIXES
Table:	AV				
Revision:	Replace the table with the following:				
	A	V			
	Pay Value	Test	Result		
		(9	%)		
		AADTT Class 2	AADTT Class 3		
		CM35 Z	or 4		
	1.05	3.2-3.8	3.2-3.8		
	1.00 + 0.1 (AV-3.0)	1.5-3.1	2.0-3.1		
	1.00 + 0.1 (4.5-AV)	3.9-6.0	3.9-6.0		
	0.75	6.1-6.5			
	(1)	< 1.5 or	< 2.0 or		
		> 6.5	> 6.0		

Section:	403.03.03 Preparatio	n of Mixture		
Part:	(C)			
Subpart:	5)			
Revision:	Add new subpart 5 a	nd Table.		
	/	nce Verification. FOR ALL 0.5-inc		
	with PG 64-22, ensu	re that the following limits are met	of exceeded to obtain appro-	val:
		Mix Design Performance	e Limits	7
	Class	Hamburg Passes ¹ (min)	KYCT Index ² (min)	7
	2	7,500	95	
	3	10,000	95	
	4	10,000	125	
	(12.5 mm). The "Ha	istance shall meet the above numbamburg Passes" value is determine	-	-
		amburg Passes" value is determine	-	-
Section:	(12.5 mm). The "Ha of the test. ² Test in accordance 501.03.019 Surface 3	amburg Passes" value is determine	-	-
Section: Part:	(12.5 mm). The "Ha of the test. ² Test in accordance	amburg Passes" value is determine with KM 64-450.	-	-
Part: Subpart:	(12.5 mm). The "Ha of the test. ² Test in accordance 501.03.019 Surface The surface Th	with KM 64-450. Folerances and Testing Surface irements	d by averaging the results fro	-
Part:	(12.5 mm). The "Ha of the test. Test in accordance 501.03.019 Surface 7 B) Ride Quality 2) Category A Required Replace the last sent	with KM 64-450. Folerances and Testing Surface irements ence in the first paragraph with the	d by averaging the results from	om both side
Part: Subpart:	(12.5 mm). The "Ha of the test. ² Test in accordance 501.03.019 Surface The surface Th	with KM 64-450. Folerances and Testing Surface irements ence in the first paragraph with the discretion, a pay deduction of \$120	d by averaging the results from	om both side
Part: Subpart: Revision:	(12.5 mm). The "Ha of the test. ² Test in accordance 501.03.019 Surface 7 B) Ride Quality 2) Category A Request Replace the last sent At the Department's capplied in lieu of cor	with KM 64-450. Folerances and Testing Surface irements ence in the first paragraph with the discretion, a pay deduction of \$120	d by averaging the results from	om both side
Part: Subpart: Revision: Section:	(12.5 mm). The "Ha of the test. ² Test in accordance 501.03.019 Surface The surface Th	with KM 64-450. Folerances and Testing Surface irements ence in the first paragraph with the discretion, a pay deduction of \$120	d by averaging the results from	om both side
Part: Subpart: Revision: Section: Part:	(12.5 mm). The "Ha of the test. ² Test in accordance 501.03.019 Surface The surface Th	with KM 64-450. Folerances and Testing Surface irements ence in the first paragraph with the discretion, a pay deduction of \$120 rective work.	d by averaging the results from	om both side
Part: Subpart: Revision: Section: Part: Subpart:	(12.5 mm). The "Ha of the test. ² Test in accordance 501.03.019 Surface The surface of the Quality 2) Category A Requirate Replace the last sent of the Department's capplied in lieu of cortain 1501.03.19 B) 3) Category B Requirates	with KM 64-450. Folerances and Testing Surface irements ence in the first paragraph with the discretion, a pay deduction of \$120 rective work.	d by averaging the results from the following: 0 per 0.1-lane-mile section makes	om both side
Part: Subpart: Revision: Section: Part:	(12.5 mm). The "Ha of the test. ² Test in accordance 501.03.019 Surface The surface of the Quality 2) Category A Requiract Replace the last sent of the Department's complied in lieu of cortain 1501.03.19 B) 3) Category B Requiract Replace the last sent of the Replace the last sent of the surface of	with KM 64-450. Folerances and Testing Surface irements ence in the first paragraph with the discretion, a pay deduction of \$120 rective work.	d by averaging the results from the following: 0 per 0.1-lane-mile section in following:	nay be
Part: Subpart: Revision: Section: Part: Subpart:	(12.5 mm). The "Ha of the test. ² Test in accordance 501.03.019 Surface The surface of the Quality 2) Category A Requirate Replace the last sent of the Department's complied in lieu of cortain 1501.03.19 B) 3) Category B Requirate Replace the last sent of the Department's contained the Last sent of the Department's contained the Last sent of the Department's contained the Department's cont	with KM 64-450. Folerances and Testing Surface irements ence in the first paragraph with the discretion, a pay deduction of \$120 rective work. irements ence in the first paragraph with the	d by averaging the results from the following: 0 per 0.1-lane-mile section in following:	nay be
Part: Subpart: Revision: Section: Part: Subpart: Revision:	(12.5 mm). The "Ha of the test. ² Test in accordance 501.03.019 Surface The surface of the Quality 2) Category A Requirate Replace the last sent. At the Department's Capplied in lieu of corrective surface the last sent. At the Department's Capplicate the last sent.	with KM 64-450. Folerances and Testing Surface irements ence in the first paragraph with the discretion, a pay deduction of \$120 rective work. irements ence in the first paragraph with the discretion, a pay deduction of \$750 work.	d by averaging the results from the following: 0 per 0.1-lane-mile section in following:	nay be
Part: Subpart: Revision: Section: Part: Subpart: Revision:	(12.5 mm). The "Ha of the test. Test in accordance 501.03.019 Surface The surface of the Quality 2) Category A Requirement of the Last sent	with KM 64-450. Folerances and Testing Surface irements ence in the first paragraph with the discretion, a pay deduction of \$120 rective work. irements ence in the first paragraph with the discretion, a pay deduction of \$750 work.	d by averaging the results from the following: 0 per 0.1-lane-mile section in following:	nay be
Part: Subpart: Revision: Section: Part: Subpart: Revision:	(12.5 mm). The "Ha of the test. ² Test in accordance 501.03.019 Surface The surface of the Quality 2) Category A Required Replace the last sent. At the Department's complied in lieu of corrective to the last sent. At the Department's complete the last sent.	with KM 64-450. Folerances and Testing Surface irements ence in the first paragraph with the discretion, a pay deduction of \$120 rective work. irements ence in the first paragraph with the discretion, a pay deduction of \$750 work. TION entence with the following:	following: 0 per 0.1-lane-mile section mage following: per 0.1-lane-mile section mage following:	nay be
Part: Subpart: Revision: Section: Part: Subpart: Revision:	(12.5 mm). The "Ha of the test. ² Test in accordance 501.03.019 Surface The surface of the Quality 2) Category A Required Replace the last sent. At the Department's complied in lieu of corrective to the last sent. At the Department's complete the last sent.	with KM 64-450. Folerances and Testing Surface irements ence in the first paragraph with the discretion, a pay deduction of \$120 rective work. irements ence in the first paragraph with the discretion, a pay deduction of \$750 work.	following: 0 per 0.1-lane-mile section mage following: per 0.1-lane-mile section mage following:	nay be

Section:	603.03.05 Drainage
Revision:	In the eighth (8th) paragraph, remove "type IV" from the fabric references in the first (1st) and
	third (3rd) sentences.
Section:	607.03.02
Part:	(a) Prequalification
Revision:	Remove the following item to the list entitled "Fabricators having SBR, IBR, ABR, or CPT
	certification may fabricate the following":
	· Armored Edges
Section:	607.03.02
Part:	(a) Prequalification
Revision:	add the following items to the list entitled "AISC certification not required for the following":
	· Armored Edges or joints with a nominal width of 4 inches or less
	· Railing System Type II
Section:	609.05 PAYMENT
Revision:	add the following line to the table "Schedule for Adjusted Quantity for Depth of Cover
	Deficiency"
	Depth of Cover Deficiency (inches) +0.26 to +0.50 ⁽⁴⁾
	Quantity Adjustment Factor 0.06
Section:	609.05 PAYMENT
Part:	Note (4) under "Schedule for Adjusted Quantity for Depth of Cover Deficiency"
Revision:	Replace note (4) with the following:
	Quantity Adjustment Factor only applies if the Contractor elects to have the bridge deck cored as
	per KM 64-313. If the Contractor accepts adjustment based on the pachometer readings, this
	Quantity Adjustment Factor is 0.00.
Section:	615 PRECAST THREE SIDED STRUCTURES
Revision:	Insert complete Section 615
Section:	701.05 PAYMENT
Revision:	Remove the following from the list of pay items:
	02600 Fabric-Geotextile Type IV for Pipe Square Yard (2)
Section:	701.05 PAYMENT
Revision:	Add the following to the list of pay items:
	02600 Fabric-Geotextile Class 2 for Pipe Square Yard (2)
Section:	701.05 PAYMENT
Revision:	Replace "Type IV" in the item name in note (2) with "Class 2"
Section:	715.02.07 "Pop" Fasteners
Revision:	Remove this section in its entirety.
Section:	715.01 DESCRIPTION
Revision:	Replace the second sentence with the following:
	Panel Signs may be ground mounted, overhead structure mounted, or bridge mounted signs.

Section:	715.02.03 Steel Reinforcement
Revision:	Change section reference from 602 to 811.
Section:	715.03 CONSTRUCTION
Revision:	Remove all but the first paragraph from this Section. 715.03 will now read as follows:
	The Department may inspect fabrication and erection work. The Department will perform a day
	and night inspection after the installation is complete.
Section:	715.03.01 Location
Revision:	Remove the first and fourth sentences from this Section, and insert 'to the plans' into the final
	sentence. The Section will now read as follows:
	Consider sign locations specified in the Plans as approximate only.
	Determine the exact location for each sign and obtain the Engineer's approval. Center overhead
	signs over the lane or lanes to which they apply.
	Allow for differences in elevation across the full shoulder width, as specified in the Plans,
	in maintaining the required 18-foot minimum vertical clearance to the bottom of the lowest
	parts of the signs or supports for overhead signs. Submit all proposed revisions to the plans in
	writing to the Engineer for written approval.
Section:	715.03.02 Messages
Revision:	Delete entire Section
Section:	715.03.03 Attachment
Revision:	Renumber as 715.03.02 Attachment
Section:	715.03.02 Attachment
Revision:	Replace section with the following:
	Letters, symbols, numbers, and borders are to be attached to the sign face using the 'direct
	applied' method.
Section:	715.03.04 Shields
Revision:	Delete entire Section
Section:	715.03.05 Covering
Revision:	Renumber as 715.03.03 Covering
Section:	715.03.06 Shop Drawings
Revision:	Renumber as 715.03.04 Shop Drawings
Section:	715.03.07 Fabrication
Revision:	Renumber as 715.03.05 Fabrication
Section:	715.03.08 Footings, Bases, and Pedestals
Revision:	Renumber as 715.03.06 Footings, Bases, and Pedestals
Section:	715.03.06 Footings, Bases, and Pedestals
Revision:	Add the following as the first sentence in the third paragraph:
	Use Class A concrete according to Subsection 601.03.
Section:	715.03.09 Sign Beams and Supports
Revision:	Renumber as 715.03.07 Sign Beams and Supports

Section:	715.03.07 Sign Beams and Supports
Revision:	Remove "and Type "B" " from the third sentence in the first paragraph.
Section:	715.03.07 Sign Beams and Supports
Part:	B)
Revision:	Remove part B) Type B Beam
Section:	715.03.07 Sign Beams and Supports
Part:	C) Type C Beam
Revision:	Change part number as follows:
	B) Type C Beam
Section:	715.03.07 Sign Beams and Supports
Part:	C) Type D Breakaway Supports
Revision:	New part C) after removal of Type B Beam from list with text as follows:
	Specifications for Type D breakaway supports are listed on the details sheet for Type "D"
	supports.
Section:	715.03.10 Bridge Mounting for Signs
Revision:	Renumber section as 715.03.08 Bridge Mounting for Signs
Section:	715.03.11 Mounting Signs
Revision:	Renumber section as 715.03.09 Bridge Mounting for Signs
Section:	715.03.10 Logo Signs
Revision:	Insert new section 715.03.10 Logo Signs, with text as follows:
	Unless directed in the project plans, existing logo panel signs are to be kept in service during construction. Contact the logo contractor if signs are to be out of service for more than one day. Temporary installations shall be on square wood posts (with the holes drilled in the bottom, per the detail sheet, for locations not protected by guardrail, barrier wall, etc.).
Section:	715.04.03 Sign Supports
Revision:	Replace the second paragraph with the following:
	The Department will not measure clearing and grubbing or excavation for payment and will
	consider them incidental to this item of work.
Section:	715.04.06 Sign Panels
Revision:	add the following as the second paragraph in this section:
	The Department will not measure temporary panel signing for payment and will consided them
	incidental to this item of work.
Subsection:	716.03.10 Electrical Junction Box
Part:	B) Filter Fabric
Revision:	rename part B) to the following: Geotextile Fabric
Subsection:	716.03.10 Electrical Junction Box
Part:	B) Geotextile Fabric
Revision:	Replace the first sentence the part with the following:
	Before the installation of the #57 aggregate and junction box, the contractor shall install
	geotextile fabric for subsurface drainage and separation in the bottom of hole.
	9

Subsection:	725.04.0	725.04.06 Concrete, Class AA (for pads)								
Revision:	Replace this subsection with the following:									
	The Dep	The Department will not measure the quantities of Concrete Class AA, excavation, or steel								
	reinforc	ement for p	payment, and will	consider them incidental to Cras	h Cushion Type VI	I, Type				
	VI, or T	VI, or Type VI-T.								
Subsection:	725.05	725.05 PAYMENT								
Revision:	Remove	the follow	ring from the list of	of pay items:						
	08104	Concrete,	Class AA Cu	bic Yard						
Subsection:	801.01	REQUIRE	MENTS							
Part:	3)									
Subpart:	a)									
Revision:	Replace	the second	sentence with the	e following:						
	Ensure 1	that the loss	s on ignition of the	e fly ash does not exceed 4.0 per	cent.					
Section:	804.04.0	04 Require	ments for Combin	ed Aggregates						
Revision:	Replace	the table v	with the following:	:						
	FINE AGGREGATE CONSENSUS PROPERTY									
				REQUIREMENTS						
		AADTT Class	Design AADTT	Uncompacted Void Content of Fine Aggregate (Percent), ⁽¹⁾ Minimum	Sand Equivalent (Percent), Minimum					
		2	<600	40.0	40					
		3	600 to 2999	43.0	45					
		4	>3000	45.0	50					
Section:	804.01	GENERAL	···							
Revision:	Replace	the second	l paragraph with tl	he following:						

The Department's List of Approved Materials includes the Aggregate Source List and the list of Class A and Class B Polish-Resistant Aggregate Sources, the Concrete Aggregate Restriction

List, Lightweight Aggregate Source List, and Microsurface Aggregate Source List.

Section:	804	804.04.05 Microsurface.						
Revision:	Mod	dify the Table	e as follows:					
	3 N N N	ieve Size /8 inch Jo. 4 Jo. 8 Jo. 16 Jo. 30 Jo. 50 Jo. 100 Jo. 200	Type II % Passing 100 90-100 65-9060-90 45-7040-70 30-5025-50 18-3015-30 10-21 5-15		Type III % Passing 100 70-9070-100 45-70 28-50 19-34 12-25 7-18 5-15	± 5% ± 5% ± 5% ± 5% ± 5% ± 4% ± 3% ± 2%		
Section:	805	.01 GENERA	A.I					
Revision:	ı		nd paragraph with t	he following:				
Section: Revision:	ı	lace the table	TURES AND SEAD with the following COARSE AGGE	:		OPERTY		
			T ====================================					
		AADTT Class	Design AADTT	(Pero	larity	Elongated ^(a) (Percent), maximum		
		2	<600	85	80	10		
			The second secon	65	80	Sent Member		
		3	600 to 2999	95	90	10		
		4	≥ 3000	100	100	10		
		⁽¹⁾ Criterion	based on a 5:1 r					

Section: Revision: 806.03.01 General Requirements

Revise the table with the following edited Dynamic Shear values:

1.0 21.11	DER REQUIREM					
		PG 58-28 (PG	588-28)			
Test	Specification	100% Pay	90% Pay	80% Pay	70% Pay	50% Pay
Original Binder			Ì	ĺ		Ì
Dynamic Shear, G*/sinδ	1.00 kPa Min.	1.00-0.95	0.94-0.90	0.89-0.85	0.84-0.80	< 0.80
Viscosity	3 Pa·s					
RTFO Residue						
Mass Loss, %	1.00 Max.	1.01-1.10	1.11-1.20	1.21-1.30	1.31-1.40	> 1.40
MSCR						
$J_{nr3.2}$, Max.	4.5 kPa ⁻¹	< 4.7	4.71-4.75	4.76-4.80	4.81-4.85	≥ 4.86
J _{nr_diff} , Max	75 %					
PAV Aging						
BBR		Š.				
Creep Stiffness	300 MPa Max.	300-315	316-330	331-345	346-360	> 360
m-value	0.300 Min.	0.290-0.300	0.285-0.289	0.280-0.284	0.275-0.279	< 0.274
Dynamic Shear,	5,000 kPa Max.	0-5,200	5,101-5,300	5,301-5,400	5,401-5,500	> 5,501
G*sinδ @ 25 °C (2)	6,000 kPa Max.	0-6,200	6,201-6,300	6,301-6,400	6,401-6,500	> 6,50

DO (1.00 DO (10.00)

		PG 64-22 (PC	6 64S-22)			20-
Test	Specification	100% Pay	90% Pay	80% Pay	70% Pay	50% Pay(1)
Original Binder				İ		Ì
Dynamic Shear, G*/sinδ	1.00 kPa Min.	1.00-0.95	0.94-0.90	0.89-0.85	0.84-0.80	< 0.80
Viscosity	3 Pa·s					
RTFO Residue						
Mass Loss, %	1.00 Max.	1.01-1.10	1.11-1.20	1.21-1.30	1.31-1.40	> 1.40
MSCR J _{nr3.2} , Max.	4.5 kPa ⁻¹	< 4.7	4.71-4.75	4.76-4.80	4.81-4.85	≥ 4.86
J _{nr_diff,} Max. PAV Aging	75 %					
BBR Creep Stiffness	300 MPa Max.	300-315	316-330	331-345	346-360	> 360
m-value	0.300 Min.	0.290-0.300	0.285-0.289	0.280-0.284	0.275-0.279	< 0.274
Dynamic Shear, G*sinδ ⁽²⁾	5,000 kPa Max. 6,000 kPa Max.	0-5,200 0-6,200	5,201-5,300 6,201-6,300		5,401-5,500 6,401-6,500	> 5,501 > 6,501

Section:	806.03.01 General Requirements.							
Revision:	Revise the Table with the following corrected values:							
	RTFO Residue							
	Mass Loss, %	1.00 Max.	1.01-1.10	1.11-1.20	1.21-1.30	1.31-1.40	> 1.40	
	MSCR							
	$J_{nr3.2}$, Max	0.5 kPa ⁻¹	< <u>4.70.7</u>	4 .71- 4 .75 0.71-0.75	4 .76- 4 .80 0.76- 0.80	4.81- 4.85 <u>0.81-</u> 0.85	≥ 4.86 0.86	
	J _{nr. diff} , Max	75 %						
Section:	805.03.02 Physical 1	Properties						
Revision:	Replace the first 2 lines in this section with the following:							
	Wear (Except Slag, Granite, and Sandstone)			40% maximum				
	Wear (Granite and Sandstone)			50% maximum				

Section:	814.06 MATERIALS FOR END TREATMENTS			
Part:	A) Anchorage Systems			
Revision:	Revise the minimum breaking strenght to be 42,800, and replace reference to AASHTO M 30,			
	Class C with AASHTO M 30, Class A.			
Section:	816.07.03 Wood Posts and Braces			
Revision:	Delete the last sentence of the first paragraph. The AWPA C 2 standard no longer exists.			
Section:	820.01 Requirements			
Revision:	Replace the second paragraph with the following:			
	Treated timber poles shall meet the general requirements of this subsection and shall be treated			
	according to AASHTO M133 in accordance with requirements of the current AWPA standards.			
Section:	820.01 Requirements			
Revision:	Delete the second sentence of the third paragraph.			
Section:	830.02.01 Delineator Sheeting			
Part:	A) Barrier Wall Delineator			
Revision:	Replace text with the following:			
	Use retroreflective sheeting conforming to ASTM D 4956, Type XI, Class 1.			
Section:	830.02.01 Delineator Sheeting			
Part:	B) Guardrail Delineator			
Revision:	Replace text with the following:			
	Use retroreflective sheeting conforming to ASTM D 4956, Type XI, Class 1.			
Section:	830.02.01 Delineator Sheeting			
Part:	C) Delineator Post			
Revision:	Replace text with the following:			
	Use retroreflective sheeting conforming to ASTM D 4956, Type XI, Class 1.			
Section:	830.02.03 Drum Sheeting			
Revision:	Replace text with the following:			
	Use retroreflective sheeting conforming to ASTM D 4956. Use approved types for necessary			
	colors on the Department's List of Approved Materials.			
Section:	830.02.04 Cone and Tubular Marker Sheeting			
Revision:	Replace text with the following:			
	Use retroreflective sheeting conforming to ASTM D 4956. Use approved types for necessary			
	colors on the Department's List of Approved Materials.			
Section:	830.02.06 Permanent Sign Sheeting			
Revision:	Replace text with the following:			
	Use retroreflective sheeting conforming to ASTM D 4956, Type XI, Class 1.			
Subsection:	834.07.05 Geotextile Filter Fabric Type IV			
Revision:	Change the subsection title to the following: Geotextile Fabric			
Section:	837.03 APPROVAL			
Revision:	In the first sentence, replace 'AASHTO T-250' with 'KM 64-268'			

Section:	837.03 APPROVAL
Revision:	Replace the second sentence with the following:
	The Department will sample and evaluate for approval each shipment of each lot of
	thermoplastic material delivered for use per contract prior to installation of the thermoplastic
	material.
Section:	837.03.01 Composition
Revision:	Add the following sentence to the end of the paragraph:
	Manufacturers are to produce extruded thermoplastic in compliance with the values listed in
	Table 1.
Section:	837.03.01 Composition
Revision:	Label Composition table as new subsection '837.03.02 Table 1'.
Section:	837.03.02 Physical Characteristics
Revision:	Renumber subsection as 837.03.03
Section:	837.06 MANUFACTURER'S TESTING
Revision:	In the first sentence, replace 'AASHTO T-250' with 'KM 64-268'
Section:	837.09 ACCEPTANCE OF NON-SPECIFICATION COMPLIANT THERMOPLASTIC
Revision:	Add new subsection with the following text:
	The Department may accept thermoplastic found to be in non-conformance to the Specification Acceptance Range at a reduction in pay, see Table 2. Thermoplastic with analytical test results not conforming to the Specification Acceptance Range but within the Acceptance Range with Deduction may be accepted for incorporation into the project with applicable reductions in pay. Deductions are cumulative to a maximum of 60% reduction in pay applied to the contract unit bid price for the thermoplastic. Thermoplastic with three (3) or more analytical tests resulting in non-conformance to the Specification Acceptance Range or any analytical test result exceeding the Acceptance Range with Deduction will be rejected and removed from the project. Do not allow transfer of thermoplastic materials between projects that have analytical test results in the

Section:	837.10 Table 2					
Revision:	Add new subsection titled 'Table 2' with the following table:					
	027 10 T-11- 2					
	837.10 Table 2.	MOPLASTIC PRICE	ADIUSTMENT SCH	EDIT E		
		Specification	Acceptance Range	Deduction Applied to Unit		
	Analytical Test	Acceptance Range	with Deduction	Cost		
	Binder, %	6 18.0 min.	16.0 -17.9	50%		
	Glass Beads % (Premixed		28-30	20%		
	Titanium Dioxide % for white		9.0 -9.9	20%		
	Calciun Carbonate and Inert Fillers fo	i				
	white					
	Calciun Carbonate and	1				
	Inert Fillers fo Yellow					
	Heavy Metal Conten					
	Colo		6.0 ΔΕ*- 8.0 ΔΕ*	10%		
Subsection:	844.01 FLY ASH REC	844.01 FLY ASH REQUIREMENTS				
Revision:	Replace the first paragraph with the following, in order to increase the loss on ignition t percent:					
	*	morata mivturas os o so	marata ingradient con	form to ASTM C 61		
	For fly ash added to concrete mixtures as a separate ingredient, conform to ASTM C 613					

F or Class C, except ensure that the loss on ignition does not exceed 4.0 percent or 6.0 percent

for fly ash receiving an approved chemical treatment.

SECTION 615 PRECAST THREE SIDED STRUCTURES

- **615.01 DESCRIPTION.** This work shall consist of constructing precast concrete three sided units for culverts, storm sewers, tunnels, arch bridges, etc. in accordance with these specifications and in reasonably close conformity with the lines, grades, design and dimensions shown on the plans or as established by the Engineer. In situations where two or more specifications apply to this work, the most stringent requirements shall govern.
- **615.02 TYPES.** Precast reinforced concrete units manufactured in accordance with this specification shall be designated by span and rise. Precast reinforced concrete endwalls manufactured in accordance with this specification shall be designated by length and height.
- **615.03 MATERIALS CONCRETE.** The concrete for the structures shall be airentrained when installed in areas subject to freeze-thaw conditions, composed of Portland cement, fine and coarse aggregates, admixtures and water. Air-entrained concrete shall contain 6 ± 2 percent air. The air entraining admixture shall conform to AASHT0 M154.
- **615.03.01 Portland Cement.** Shall conform to the requirements of ASTM Specifications C150-Type I, Type II, or Type III cement.
- **615.03.02 Coarse Aggregate.** Shall consist of stone having a maximum size of 1 inch. Aggregate shall meet requirements for ASTM C33.
- **615.03.03 Water Reducing Admixture.** The manufacturer may submit for approval by the Engineer, a water-reducing admixture for the purpose of increasing workability and reducing the water requirement for the concrete.
- **615.03.04 Calcium Chloride.** The addition to the mix of calcium chloride or admixtures containing calcium chloride will not be permitted.
- **615.04 MATERIALS STEEL REINFORCEMENT AND HARDWARE.** All reinforcing steel for the structures shall be fabricated and placed in accordance with the detailed shop drawings submitted by the manufacturer.
- **615.04.01 Steel Reinforcement.** Reinforcement shall consist of welded wire fabric conforming to ASTM Specification A 185 or A 497, or deformed billet steel bars conforming to ASTM Specification A 615, Grade 60. Longitudinal distribution reinforcement may consist of welded wire fabric or deformed billet-steel bars.
- **615.04.02 Hardware.** Inserts for endwall connections shall be AISI Type 304 stainless steel, F-58 Expanded Coil inserts. Coil rods and nuts used in endwall connections shall be AISI Type 304 stainless steel. Washers used in endwall connections shall be AISI Type 304 stainless steel plate washers. Or Equals

Reinforcing bar splices shall be made using the Dowel Bar Splicer System, and shall consist of the Dowel Bar Splicer (DB-SAE) and Dowell-In(DI) or equal system.

Hook Bolts used in endwall connections shall be ASTM A 307.

615.05 MANUFACTURE.

615.05.01 Mixture. The aggregates, cement and water shall be proportioned and mixed in a batch mixer to produce a homogeneous concrete meeting the strength requirements of this specification. The proportion of Portland cement in the mixture shall not be less than 564 pounds (6 sacks) per cubic yard of concrete.

- **615.05.02 Curing.** The precast concrete units shall be cured for a sufficient length of time so that the concrete will develop the specified compressive strength in 28 days or less. Any one of the following methods of curing or combinations thereof shall be used:
 - **A) Steam Curing.** The units may be low pressure, steam cured by a system that will maintain a moist atmosphere.
 - **B)** Water Curing. The units may be water cured by any method that will keep the sections moist.
 - C) Membrane Curing. A sealing membrane conforming to the requirements of ASTM Specification C 309 may be applied and shall be left intact until the required concrete compressive strength is attained. The concrete temperature at the time of application shall be within \pm 10 degrees F of the atmospheric temperature. All surfaces shall be kept moist prior to the application of the compounds and shall be damp when the compound is applied.
- **615.05.03 Forms.** The forms used in manufacturing shall be manufactured steel forms and accurate to maintain the structure dimensions within the permissible variations given in Section 7 of these specifications. All casting surfaces shall be of a smooth material.
- **615.05.04 Handling.** Handling devices or holes shall be permitted in each unit for the purpose of handling and setting.
- **615.05.05 Storage.** The precast elements shall be stored in such a manner to prevent cracking or damage. The units shall not be moved until the concrete compressive strength has reached a minimum of 2500 psi, and they shall not be stored in an upright position until the concrete compressive strength is a minimum of 4,000 psi.
- **615.05.06 Weep holes.** Place weep holes consisting of 4-inch pipe or formed to 4 inches in diameter in each precast unit. Fabric wrapped perforated pipe drains may be used in retaining walls in place of weep holes. Place the outlet invert elevation of weep holes in box culverts 4 inches above the flowline of the structure. Raise weep holes to accommodate significant silting when the Engineer directs. Make adequate provisions for thorough drainage of backfill and embankment according to Subsection 603.03.

615.06 DESIGN.

615.06.01. Obtain the precast concrete 3-sided structure and endwalls from a preapproved manufacturer list maintained by the Division of Highway Design. The precast element dimension and reinforcement details shall be as prescribed in the plan and the shop drawings provided by the manufacturer, subject to the provisions of Section 7, below. The minimum concrete compressive strength shall be as shown on the shop drawings. The minimum steel yield strength shall be 60,000 psi, unless otherwise noted on the shop drawings.

The manufacturer shall submit a pdf copy of the Working Drawings, Shop Drawings, and Structural Design Calculations to the Department for review and approval prior to manufacturing the precast 3-Sided units or endwalls.

615.06.02. The precast elements shall be designed in accordance with KYHL-93. A minimum of one foot of cover is required. "Cover" is defined as the area from the top of structure to the top of finished roadway, along the entire length of structure over the driving lanes and shoulder. (Unless noted otherwise on the shop drawings, designed accordingly, and approved by this Department). The ends of units shall be normal to walls and centerline except exposed edges shall be beveled \(^3\)4 inch.

615.06.03 Placement of Reinforcement in Precast 3-Sided Units. The cover of concrete over the outside circumferential reinforcement shall be 2 inches minimum. The

cover of concrete over the inside circumferential reinforcement shall be 1 1/2 inches minimum, unless otherwise noted on the shop drawings. The clear distance of the end circumferential wires shall not be less than one inch nor more than two inches from the ends of each section. Reinforcement shall be assembled utilizing single or multiple layers of welded wire fabric (not to exceed 3 layers), supplemented with a single layer of deformed billet-steel bars, when necessary. Welded wire fabric shall be composed of circumferential and longitudinal wires meeting the spacing requirements of 615.06.06, below, and shall contain sufficient longitudinal wires extending through the vault unit to maintain the shape and position of the reinforcement. Longitudinal distribution reinforcement may be welded wire fabric or deformed billet-steel bars and shall meet the spacing requirements of 615.06.06, below. The ends of the longitudinal distribution reinforcement shall be not more than 3 inches and not less than 1 1/2 inches from the ends of the unit.

615.06.04 Placement of Reinforcement for Precast Endwalls. The cover of concrete over the longitudinal and transverse reinforcement shall be 2 inches minimum. The clear distance from the end of each precast element to the end transverse reinforcing steel shall not be less than one inch nor more than two inches. Reinforcement shall be assembled utilizing a single layer of welded wire fabric, or a single layer of deformed billet-steel bars. Welded wire fabric shall be composed of transverse and longitudinal wires meeting the spacing requirements of 615.06.07, below, and shall contain sufficient longitudinal wires extending through the element to maintain the shape and position of the reinforcement. Longitudinal reinforcement may be welded wire fabric or deformed billet-steel bars and shall meet the spacing requirements of 615.06.07, below. The ends of the longitudinal reinforcement shall be not more than 3 inches and not less than 1 1/2 inches from the ends of the walls.

615.06.05 Bending of Reinforcement for Precast 3-Sided Units. The outside and inside circumferential reinforcing steel for the corners of the structure shall be bent to such an angle that is approximately equal to the configuration of the structures outside corner.

615.06.06 Laps, Welds, and Spacing for Precast 3-Sided Units. Tension splices in the circumferential reinforcement shall be made by lapping. Laps may not be tack welded together for assembly purposes. For smooth welded wire fabric, the overlap shall meet the requirements of AASHTO 2012 Bridge Design Guide Section 5.11.2.5.2 and AASHTO 2012 Bridge Design Guide Section 5.11.2.5.1 and AASHTO 2012 Bridge Design Guide Section 5.11.6.2. The overlap of welded wire fabric shall be measured between the outer most longitudinal wires of each fabric sheet. For deformed billet-steel bars, the overlap shall meet the requirements of AASHTO 2012 Bridge Design Guide Section 5.11.2.1. For splices other than tension splices, the overlap shall be a minimum of 12" for welded wire fabric or deformed billet-steel bars. The spacing center to center of the circumferential wires in a wire fabric sheet shall be no less than 2 inches and no more than 4 inches. The spacing center to center of the longitudinal wires shall not be more than 8 inches. The spacing center to center of the longitudinal distribution steel for either line of reinforcing in the top slab shall be not more than 16 inches.

615.06.07 Laps, Welds, and Spacing for Precast Endwalls. Splices in the reinforcement shall be made by lapping. Laps may not be tack welded together for assembly purposes. For smooth welded wire fabric, the overlap shall meet the requirements of AASHTO 2012 Bridge Design Guide Section 5.11.2.5.2 and AASHTO 2012 Bridge Design Guide Section 5.11.6.3. For deformed welded wire fabric, the overlap shall meet the requirements of AASHTO 2012 Bridge Design Guide Section 5.11.2.5.1 and AASHTO 2012 Bridge Design Guide Section 5.11.2.5.1 he overlap shall meet the requirements of AASHTO 2012 Bridge Design Guide Section 5.11.2.1. The

spacing center-to-center of the wire fabric sheet shall not be less than 2 inches or more than 8 inches.

615.07 PERMISSIBLE VARIATIONS.

615.07.01 Precast 3-Sided Units.

- **A) Internal Dimensions.** The internal dimension shall vary not more than 1% from the design dimensions nor more than 1-1/2 inches whichever is less. The haunch dimensions shall vary not more than 3/4 inch from the design dimension.
- **B)** Slab and Wall Thickness. The slab and wall thickness shall not be less than that shown in the design by more than 1/4 inch. A thickness more than that required in the design shall not be cause for rejection.
- C) Length of Opposite Surfaces. Variations in laying lengths of two opposite surfaces of the vault unit shall not be more than 1/2 inch in any section, except where beveled ends for laying of curves are specified by the purchaser.
- **D)** Length of Section. The underrun in length of a section shall not be more than 1/2 inch in any vault unit.
- **E)** Position of Reinforcement. The maximum variation in position of the reinforcement shall be $\pm 1/2$ inch. In no case shall the cover over the reinforcement be less than 1 1/2 inches for the outside circumferential steel or be less than 1 inch for the inside circumferential steel as measured to the external or internal surface of the vault. These tolerances or cover requirements do not apply to mating surfaces of the joints.
- **F)** Area of Reinforcement. The areas of steel reinforcement shall be the design steel areas as shown in the manufacturer's shop drawings. Steel areas greater than those required shall not be cause for rejection. The permissible variation in diameter of any reinforcement shall conform to the tolerances prescribed in the ASTM Specification for that type of reinforcement.

615.07.02 Endwalls.

- A) Wall Thickness. The wall thickness shall not vary from that shown in the design by more than 1/2 inch.
- B) Length/ Height of Wall sections. The length and height of the wall shall not vary from that shown in the design by more than 1/2 inch.
- C) Position of Reinforcement. The maximum variation in the position of the reinforcement shall be \pm 1/2 inch. In no case shall the cover over the reinforcement be less than 1 1/2 inches.
- D) Size of Reinforcement. The permissible variation in diameter of any reinforcing shall conform to the tolerances prescribed in the ASTM Specification for that type of reinforcing. Steel area greater than that required shall not be cause for rejection.

615.08 TESTING AND INSPECTION.

- 615.08.01 Type of Test Specimen. Start-up slump, air content, unit weight, and temperature tests will be performed each day on the first batch of concrete. Acceptable start-up results are required for production of the first unit. After the first unit has been established, random acceptance testing is performed daily for each 50 yd3 (or fraction thereof). In addition to the slump, air content, unit weight, and temperature tests, a minimum of one set of cylinders shall be required each time plastic property testing is performed.
- **615.08.02 Compression Testing.** Cylinders shall be made and tested as prescribed by the ASTM C 39 Specification.
- 615.08.03 Acceptability of Cylinder Tests. When the average compressive strength of all cylinders tested is equal to or greater than the design compressive strength, and not more than 10% of the cylinders tested have a compressive strength less than the design concrete strength, and no cylinder tested has a compressive strength less than 80% of the design compressive strength, then the lot shall be accepted. When the compressive strength of the cylinders tested does not conform to this acceptance criteria, the acceptability of the lot may be determined as described in section 8.4, below.
- **615.09 JOINTS.** Precast 3-sided units shall be produced with flat butt ends. The ends of the units shall be such that when the sections are laid together they will make a continuous line with a smooth interior free of appreciable irregularities, all compatible with the permissible variations in Section 7, above. The joint width shall not exceed 3/4 inches. Flattop units with less than 2 ft. of cover shall be produced with a minimum 4" deep by 1.5" wide key way joint. Mortar in accordance with section 15.2 shall be placed in the keyway.
- When the installed height of cover measures 2.0-feet or less, the precast 3-sided end units shall be connected by tie plates to the adjacent interior unit.
- **615.10 WORKMANSHIP AND FINISH.** The precast units and endwalls shall be substantially free of fractures. The ends of the units shall be normal to the walls and centerline of the section, within the limits of the variations given in section 7, above, except where beveled ends are specified. The faces of the endwalls and units shall be parallel to each other, within the limits of variations given in section 7, above. The surface of the precast elements shall be a smooth steel form or troweled surface. Provide an ordinary surface finish.
- **615.11 REPAIRS.** Precast elements may be repaired, if necessary, because of imperfections in manufacture or handling damage and will be acceptable if, in the opinion of the purchaser, the repairs are sound, properly finished and cured, and the repaired section conforms to the requirements of this specification.
- **615.12 INSPECTION.** The quality of materials, the process of manufacture, and the finished structures shall be subject to Department guidelines, specifications, manuals, and other contract documents. Units will arrive at jobsite with the "Kentucky Oval" stamped on the unit which is an indication of acceptable inspection at the production facility. Units shall be inspected upon arrival for any evidence of damage resulting from transport to the jobsite.
- **615.13 REJECTION.** The precast elements shall be subject to rejection on account of any of the specification requirements. Individual precast elements may be rejected because of any failure to meet specification and contract document requirements.
- **615.14 MARKING.** Each unit shall be clearly marked by waterproof paint. The following shall be shown on the inside of the vertical leg of the section: Unit Span, Unit Rise, Date of Manufacture, and Name or trademark of the manufacturer. Units must also be stenciled as outlined in Standard Drawing BGX-006, current revision.

615.15. CONSTRUCTION REQUIREMENTS. Perform structure excavation in accordance with Section 603 except as noted in this Section.

615.15.01 Site Preparation. Perform Structure Excavation according to Section 603. The foundation design must be in accordance with the appropriate Geotechnical Notes in the project bid documents. Construct foundations in accordance with the foundation design as determined by the Engineer.

615.15.02 Footings. The precast 3-Sided units and endwalls shall be installed on either precast or cast-in-place concrete footings. The design size and elevation of the footings shall be as determined by the Engineer based on KYHL-93, the applicable Geotechnical Notes in the project bid documents such as bearing capacity requirements, specified scour countermeasures, and minimum differential settlement tolerance. In cases where a minimum differential settlement tolerance is not specified in the bid contract documents, the minimum differential settlement tolerance for the precast 3-sided structure shall be 1-inch. A minimum three inch deep keyway shall be formed in the top surface of the precast 3-sided unit footing at least three inches clear of the inside and outside faces of the bridge units, unless specified otherwise on the plans. The completed footing surface shall be constructed in accordance with grades shown on the plans. When tested with a 10 foot straight edge, the keyway surface shall not vary more than 1/4 inch in 10 feet. If a precast concrete footing is used, the contractor shall prepare a 4 inch thick base layer of compacted granular material the full width of the footing prior to placing the precast footing. The foundations for precast concrete 3-sided units and endwalls must be connected by reinforcement to form one monolithic body. Expansion joints shall not be used in lieu of a continuous foundation. Compressive cylinders must reach 2,000psi before precast units shall be set on the foundation. Compressive cylinder strength must reach 80% design strength before backfill operations shall begin.

615.15.03 Placement of the Units and Endwalls. The manufacturer shall provide a Technical Representative. The Technical Representative shall be available onsite while the contractor is setting the precast 3-sided structure and thereafter as determined necessary by the Engineer.

The units and endwalls shall be placed as shown on the Engineer's plan drawings. Special care shall be taken in setting the elements to the true line and grade. The units and endwalls shall be set on 6" x 6" masonite or steel shims. A minimum gap of 1/2 inch shall be provided between the footing and the bottom of the unit's vertical legs or the endwall. The gap shall be filled with non-shrink cement grout (Portland cement and water or cement mortar composed of Portland cement, sand and water). If units have been set with temporary ties (cables, bars, etc.) all pieces in the current phase must be completely grouted before ties may be removed.

615.15.04 External Protection of Joints. The butt-joint made by two adjoining units shall be covered with a 7/8" x 1 3/8" preformed bituminous joint sealant and a minimum of a 9 inch wide joint wrap. The surface shall be free of dirt before applying the joint material. A primer compatible with the joint wrap to be used shall be applied for a minimum width of nine inches on each side of the joint. The external wrap material shall meet AASHTO M198. The joint shall be covered continuously from the bottom of one unit section leg, across the top of the and to the opposite unit section leg. Any laps that result in the joint wrap shall be a minimum of six inches long with the overlap running downhill.

In addition to the joints between unit units, the joint between the end unit and the endwall shall also be sealed as described above. Also, if lift holes or lift inserts are formed in the units, they shall be plugged and grouted.

During the backfilling operation, care shall be taken to keep the joint wrap in its proper location over the joint.

Internal Protection of Joints – In applications where the traveling public will be expected to cross under the structure (roadways, mixed use paths, sidewalks, etc.), or where specified in the contract documents, provide additional joint protection to ensure that the structure is water-tight. Various joint sealing details including elastomeric, urethane, or liquid sealing may be shown on the plans. Any internal joint sealing shall be performed as indicated on the shop drawings.

615.15.05 Backfill. Critical Backfill shall be clean, durable stone backfill that conform to the Structural Granular backfill requirements Section 805. Critical Backfill shall be paid as Structural Granular Backfill. The limits of Critical Backfill shall be 2 feet to the outside of each structure and from the top of the footing to 2 feet over the top of the structure for spans up to 24 feet. For spans greater than 24 feet, the limits of Critical Backfill shall be 4 feet to the outside of each structure and from the top of the footing to 2 feet over the top of the structure. Backfill shall be considered as all other replaced excavation and new embankment adjacent to the precast units and endwalls. The project construction and material specifications which include the specifications for excavation for structures and roadway excavation and embankment construction, shall apply except as modified in this section. Any backfill requirements of the manufacturer that are beyond the limits of the Department-specified critical backfill requirements shall be binding upon the Contractor but will not be measured for payment as they shall be considered incidental to the critical backfill quantity.

No backfill shall be placed against any structural elements until they have been approved by the Engineer. Backfill against external joint material or waterproofed surface shall be placed carefully to avoid damage to the waterproofing material.

Mechanical tampers or approved compacting equipment shall be used to compact all backfill and embankment immediately adjacent to each side and over the top of each precast 3-sided unit until it is covered to a minimum depth of one foot, unless the design fill height is less than 1'-0". The backfill within the Critical Backfill Zone, as defined above, shall be placed in lifts of six inches or less (loose depth). Heavy compaction equipment shall not be operated in this area or over the bridge until it is covered to a depth of one foot, unless the design fill height is less than 1'-0".

Lightweight dozers and graders may be operated over precast units having one foot of compacted cover, but heavy earth moving equipment (larger than a D-4 Dozer weighing in excess of 12 tons and having track pressures of eight psi or greater) shall require two feet of cover unless the design cover is less than two feet. In no case shall equipment operating in excess of the design live load (KYHL-93) be permitted over the precast 3-sided units unless approved by the producer. Any additional fill and subsequent excavation required to provide this minimum cover shall be made at no additional cost to the project. As a precaution against introducing unbalanced stresses on the precast 3-sided units, when placing backfill at no time shall the difference between the heights of fill on opposite sides of the vault exceed 24". Once fill heights reach two feet over the top of structure, backfill as specified in Kentucky Standard Specifications Division 200.

615.16 QUALITY ASSURANCE. The Precast Supplier shall conform to the requirements for precast structures in Section 605 and the KYTC Division of Materials Precast & Prestress Concrete Manual.

The Precast Supplier shall be listed on the KYTC Division of Materials list of Approved Precast Concrete Producers.

615.17 PAYMENT. The Department will make payment for the completed and accepted quantities under the following:

Code	Pay Item	Pay Unit
21804EN	3-Sided Culvert Linear Foot	Linear Foot
02231	Structure Granular Backfill	Cubic Yard
08100	Concrete-Class A	Cubic Yard

08150	Steel Reinforcement	Pound
08003	Foundation Preparation	See Section 603
08002	Structure Excavation Solid Rock	See Section 603
08001	Structure Excavation Common	See Section 603
02203	Structure Excavation Unclassified	See Section 603
02200	Roadway Excavation	See Subsection 204.05
02230	Embankment in Place	See Subsection 206.05

The Department will consider payment as full compensation for all work required under this section.